

Southern Link Road Stage 2

Water Management & Impact Assessment

Prepared for the City of Canning

By Urbaqua

November 2018

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1 INTRODUCTION

Undertaking planning studies that aim to improve amenity and facilitate intensification of land use, the City of Canning prepared the City Centre Structure Plan to guide implementation of town planning instruments. The Structure Plan has proposed new roads and other infrastructure to be constructed around Cannington Swamp and these have the potential to impact the wetland and threatened ecological community (TEC).

Stage 2 of the Southern Link Road proposes to connect Grey Street and the Grose Avenue / Liege Street roundabout in Cannington. The proposed route traverses along the boundary of the Cannington Swamp which is recognised for its conservation values, containing a TEC and a conservation category wetland (CCW).

This Water Management and Impact Assessment has been prepared for the City of Canning to inform the construction of the proposed road and to determine the potential impacts of road construction and other infrastructure and identify the ongoing management requirements for the TEC and surrounding land.

1.1 Location and proposed road design

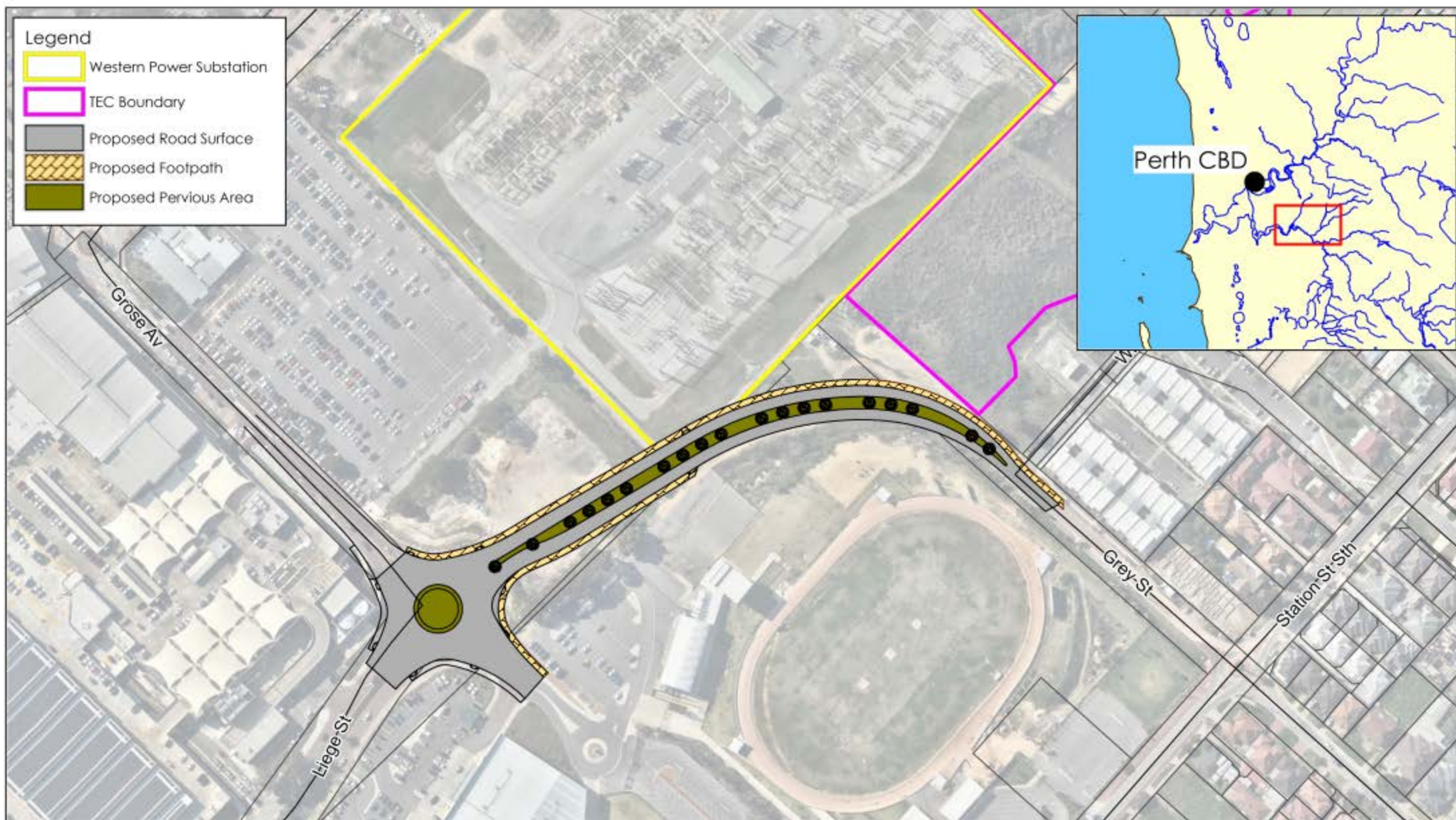
Cannington Swamp is located approximately 11 km southeast of Perth on Western Power land (behind the Cannington substation) and private land. The Cannington TEC has been identified as: Shrublands and woodlands on Muchea Limestone (endangered).

The proposed Southern Link Road stage 2 will consist of two asphalt sealed carriageways with a central landscaped area and a footpath proposed at the northern side. Details of the road design are shown in Appendix A. The road is bordering the Western Power substation and Cannington Swamp and connecting the Grose Avenue / Liege Street roundabout to Grey Street. The site location and the proposed Southern Link Road Stage 2 are illustrated in Figure 1.

This report assesses the potential environmental impacts of the road design.

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Figure 1 - Site location and proposed road layout



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2 SITE CHARACTERISTICS AND ENVIRONMENTAL ISSUES

The following review provides relevant information on the site characteristics sourced from available data and potential environmental issues.

2.1 Climate

The sites climate is typical of the Perth Metropolitan region, with warm dry summers and cooler wetter winters. The closest Bureau of Meteorology (BoM) weather station is Gosnells City located at approximately 6 km away from the site. This station has been in operation continuously since 1961.

As demonstrated in Figure 2a, there is a variation in the annual totals, ranging between 499.6mm (2010) and 1,184mm (1965). The data indicates a decreasing trend in annual and winter rainfall totals, particularly since 2000 where the annual average rainfall has decreased from 820.3mm to 729.4mm (approximately a 11.1% decrease).

Evaporation (Armadale Station), as shown in Figure 2b, is highest between November and March. A comparison of the mean monthly rainfall and evaporation totals demonstrates that the region is water limited between September and April. Between May and August rainfall exceeds evaporation.

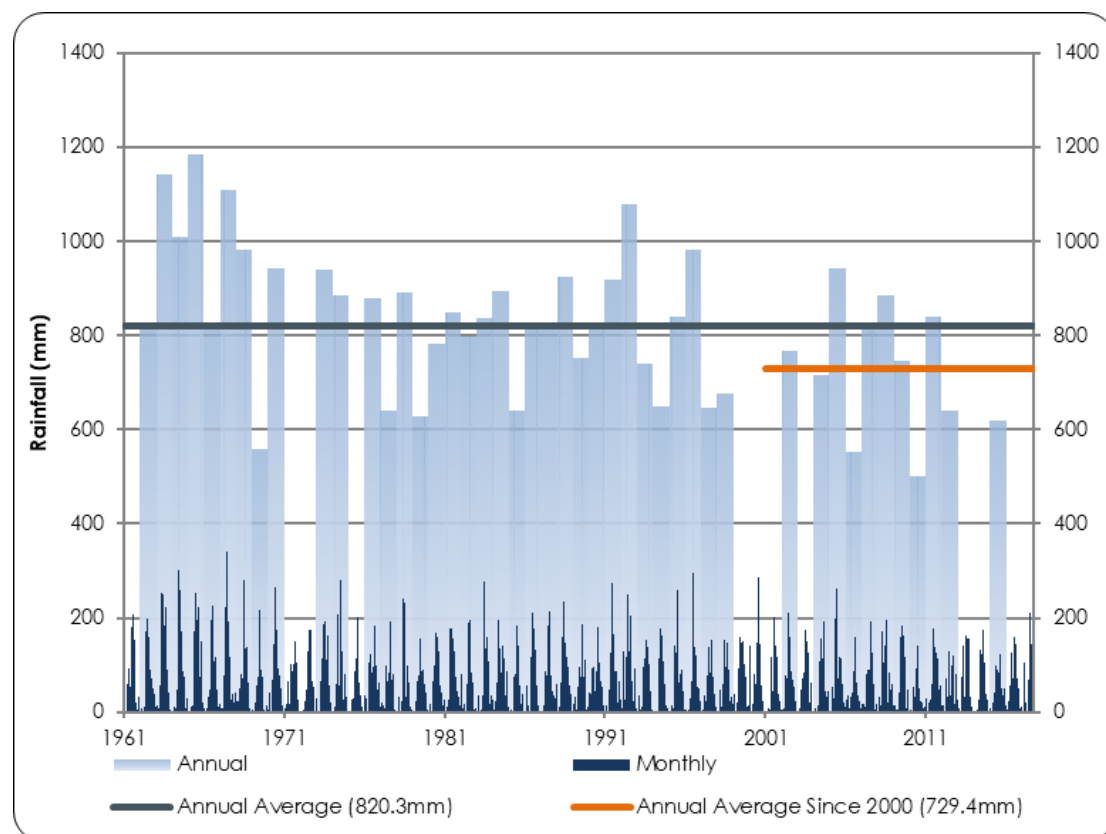


Figure 2a: Average annual and monthly climate data (station no. 9106) (BoM, 2018a)

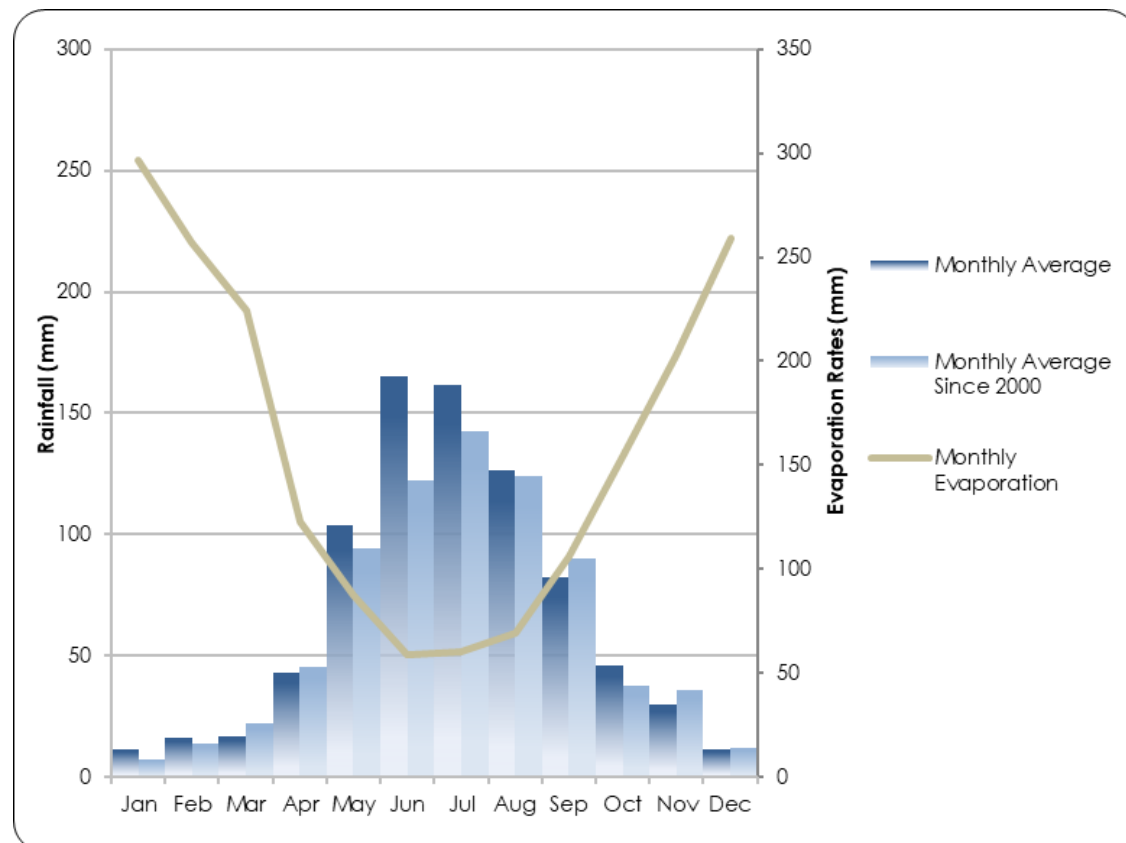


Figure 2b: Average Monthly Rainfall and Evaporation Data

2.2 Geology

The Perth Metropolitan Region 1:50,000 Environmental Geology Mapping (Jordan J. E., 1986 Armadale part sheets 2033 I and 2133 IV) defines the materials as S10, thin Bassendean Sand over Sandy Clay to Clayey Sand of the Guildford Formation of eolian origin.

The site-specific soil investigation (PB, 2005) indicates the swamp soil profile as below:

- Sandy Clay (depth of 0 – 5.5m): underlain by sand and clay layers, the clay is brown above the sandy layers and changing to dull green with depth, representing a change from oxidising to reducing condition;
- Limestone Gravel (depth of 1.5 – 4.5m): Gravel clasts are sub-angular to rounded and contain minor quartz sand fraction;
- Chalky Clay (depth of 3.5 – 5.5m): Contains coarse sand to granule cementations, occasionally range to gravel and cobble size;
- Interlayered Sand and Sandy Clay (depth of 3 – 12): sandy lenses are generally medium to coarse grained and yielded small volumes of water during air-core drilling;
- Black Clay (depth of 12 – 13m): underlain by a grey sandy unit containing some limestone cobbles and shelly material; and
- Sand with Calcareous Gravel and Shells (depth of 13 – 15m).

2.3 Biodiversity

The Southern Link Road Stage 2 construction area is entirely outside of the Cannington Swamp TEC and CCW in an area of limited cover with highly degraded vegetation. However, it is important to understand the biodiversity values present in the TEC and CCW areas so that any potential indirect impacts can be understood and managed appropriately.

2.3.1 Vegetation description

A Flora, Vegetation and Fauna Survey undertaken (Natural Area, 2016) for the TEC and CCW area identified the presence of nine vegetation types:

- Open *Casuarina obesa* Woodland;
- *Viminaria juncea* and *Melaleuca lateritia* Shrubland;
- Open *Hakea prostrata* Shrubland;
- *Melaleuca lateritia* Heathland;
- *Meeboldina* Sedgeland;
- *Baumea juncea* Sedgeland;
- Open *Bolboschoenus caldwellii* Sedgeland;
- *Verticordia densiflora* var. *densiflora* Heathland; and
- *Melaleuca raphiophylla* Woodland.

Vegetation condition was assessed in spring and ranged from Completely Degraded to Very Good, with the majority of the site (51.2%) recorded as Completely Degraded.

2.3.2 Flora

A total of 111 flora species were recorded from 43 families in the TEC and CCW area, of which:

- 42 were monocotyledons (21 native species, 21 introduced species)
- 69 were dicotyledons (36 native species, 33 introduced species).

The threatened species *Eremophila glabra* subsp. *chlorella*, and the Priority 4 species *Ornduffia submersa* were observed during the site survey activities (Natural Area, 2016).

2.3.3 Fauna

The fauna surveys identified the presence of three mammals (including the *European Red Fox* (*Vulpes Vulpes*) and the *European Rabbit* (*Oryctolagus cuniculus*) which are listed as C3 declared pests on the Western Australian Organism List (WAOL) under the Biosecurity Agriculture Management Act 2007 (WA)), 15 birds, five reptiles, four amphibians and 42 invertebrate species.

A native Bee of conservation significance, *Leioproctus douglasiellus* (a short-tongued bee) is listed by the Department of Biodiversity, Conservation and Attractions as having been recorded within the survey boundary. According to the Threatened Species Scientific Committee (2013), the Bee is closely associated with the presence of flora species *Goodenia filiformis* (Thread-leaved Goodenia) and *Anthotium junciforme*, neither of which were recorded at the site during the surveys undertaken by Natural Area in 2016. However, this does not necessarily preclude the presence of bees in the swamp area. No threatened or priority listed fauna was found during this survey activities.

2.3.4 Potential biodiversity impacts

Direct impacts

Any physical disturbance within vegetated portions of the TEC and CCW area has the potential to impact on its biodiversity values and as such should be minimised.

Indirect impacts

Any changes to the hydrological regime that supports the TEC and CCW has the potential to impact on its biodiversity values and as such should be minimised. These potential impacts are dealt with in more detail in sections 2.4 and 2.5 below.

2.4 Groundwater

The site is situated in the City of Canning subarea of the Perth groundwater area. This area consists of three aquifers, in order of depth from the surface:

- Perth-Superficial Swan Aquifer;
- Leederville Aquifer; and
- Yarragadee North Aquifer.

2.4.1 Groundwater Levels

The Perth Groundwater Atlas (DWER, 2018) indicates that the Maximum Groundwater Level (MGL) is approximately between 4 and 5 mAHD at and around the proposed road (Figure 3).

A groundwater monitoring program was undertaken by Urbaqua at Cannington TEC between July 2017 and June 2018. Monitoring included measurement of groundwater levels and groundwater sampling from the four installed bores and four existing bores. Groundwater samples were taken in July and October 2017 as per Australian Standards (AS/NZS 5667.4:1998 and AS/NZS 5667.11:1998). Location of the bores are illustrated in Figure 3.

The monthly average groundwater levels at the monitoring bores are provided in Table 1.

Table 1: Local monthly groundwater levels (mAHD)

Month	Urbaqua Bores (2017)			PB Bores (2005)	
	CS-U2	CS-U3	CS-U6	C4	C6
Jul 2017	2.65	2.94	3.92	3.31	3.40
Aug 2017	3.50	4.14	4.06	3.77	3.83
Sep 2017	3.56	4.13	3.91	3.35	3.61
Oct 2017	3.36	3.99	3.74	3.21	3.48
Nov 2017	3.00	3.21	3.27	3.01	3.19
Dec 2017	2.52	2.62	3.03	2.79	2.98
Jan 2018	2.45	2.28	3.19	2.99	3.08
Feb 2018	2.02	2.05	2.93	2.57	2.86
Mar 2018	1.56	1.77	2.84	2.49	2.74

Month	Urbaqua Bores (2017)			PB Bores (2005)	
	CS-U2	CS-U3	CS-U6	C4	C6
Apr 2018	1.67	1.59	2.98	2.49	2.77
May 2018	1.35	NA*	2.96	2.39	2.69
Jun 2018	2.26	NA	3.73	3.13	NA

*NA: NO ACCESS to the bore

2.4.2 Groundwater quality

Groundwater quality testing was undertaken from all the bores onsite in July and October 2017. The results of groundwater quality within the superficial aquifer are provided in Table 2.

pH levels were typically recorded within the guideline range for the wetlands (ANZECC & ARMCANZ, 2000) with levels slightly below 7 only at Bores CS-U1 and CS-U6. The superficial groundwater at the site is considered as Fresh with the average salinity of 3.68mg/L.

Nitrogen levels in groundwater were found to be relatively low across the site, elevated total nitrogen concentrations were identified at CS-U1 and CW1 (S) (north and north west of the site). Total Phosphorus and Ammonia levels exceeded the wetland criteria in All bores.

Table 2: Local groundwater quality (July & October 2017)

Parameter	Monitoring Results	
	Range	Average
Temperature (°C)	16.9 – 23.30	19.89
Electrical Conductivity @ 25°C (µS/ cm)	2353 - 16033	6613
Dissolved Oxygen (mg/L)	0.02 – 7.07	0.84
pH Value	6.26 – 7.78	7.19
Total Dissolved Solids @180°C (mg/L)	1541 - 10419	4300
Salinity (mg/L)	1.21 – 9.42	3.68
Total Nitrogen as N (mg/L)	0.20 – 2.80	0.69
Total Phosphorus as P (mg/L)	0.10 – 1.30	0.31
Filterable Reactive Phosphorus (mg/L)	0.01 – 0.09	0.03
Nitrite + Nitrate as N (mg/L)	0.01 – 2.00	0.25
Ammonia (mg/L)	0.02 – 0.32	0.09
Total Kjeldahl Nitrogen as N (mg/L)	0.20 – 1.80	0.48

2.4.3 Potential groundwater impacts

The construction of the road will include compaction of the soil layers beneath the road. This has the potential to make a barrier to the groundwater flow and impact the groundwater flows and levels.

2.5 Hydrology

The Cannington Swamp has been classified as a Conservation Category Wetland by the Department of Biodiversity, Conservation and Attractions in its Swan Coastal Plain geomorphic wetlands database.

2.5.1 Local drainage

The proposed road and the Cannington Swamp are located within a Water Corporation drainage catchment named as Cockram Street Main Drain (Urbaqua, 2016a) which ultimately discharges to the Liege Street constructed wetlands.

The drainage system underlying the swamp is comprised of an underground pipe network which may surcharge via raised manholes during major storm events (>20% AEP). However, this surcharge is not expected to have a significant impact on overall hydrology of the swamp as it occurs only during large storm events and have little influence on the annual hydrological cycle of the swamp (Urbaqua, 2016b).

2.5.2 Existing water balance

A water balance model has been prepared to assess the existing water levels in the swamp. Results from the model are summarised in Table 3 and Appendix B. The water balance considers the last 10 years of daily rainfall (Gosnells City station, 9106). Monthly pan evaporation rates (Armadale Station) were sourced from BOM average pan evaporation mapping (grid data) and adjusted by a pan factor of 0.75 to account for reduced evaporation that could be expected to occur from the large water bodies. Modelling assumes that the swamp is connected to the underlying aquifer with infiltration rate of 0.008m/day for Sandy Clay soils. The existing average inundation area during winter is illustrated in Figure 4.

Table 3: Existing water balance modelling inputs and results

Swamp invert (mAHD)	Base Area (ha)	Overflow level (mAHD)	Total Storage (ML)	Max GW only level (mAHD)	Winter average levels (last 10yrs) (mAHD)
3.50	0.09	4.75	65.6	3.83	4.02

2.5.3 Potential hydrological impacts

The proposed construction of the road has the potential to generate increased runoff, reduce the storage capacity and/or change the natural hydrology at the Cannington TEC.

Construction of the extended part of the road may impact the quality of stormwater. Treatment of stormwater runoff may be required to prevent export of pollutants to the swamp.

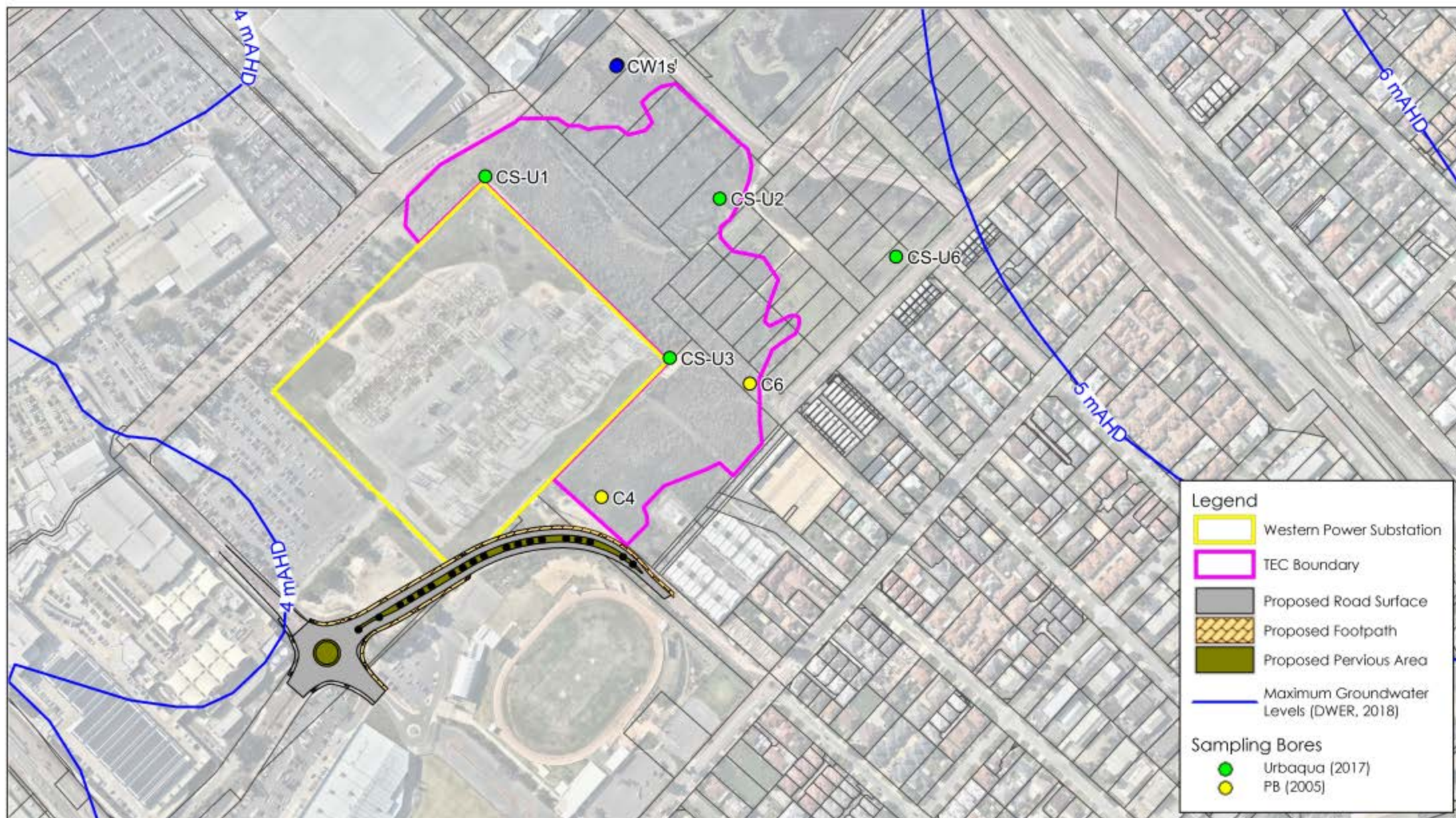
2.6 Summary of potential impacts

The identified potential impacts to the Cannington TEC following the road construction are:

- Excavation and filling works – impact on wetland hydrology and water levels;
- Road drainage discharge treatment – impacts to downstream water quality;
- Road compaction – impacts to groundwater flows/levels; and
- Clearing and physical disturbance – impact on flora/fauna and biodiversity values.

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Figure 3 - Groundwater



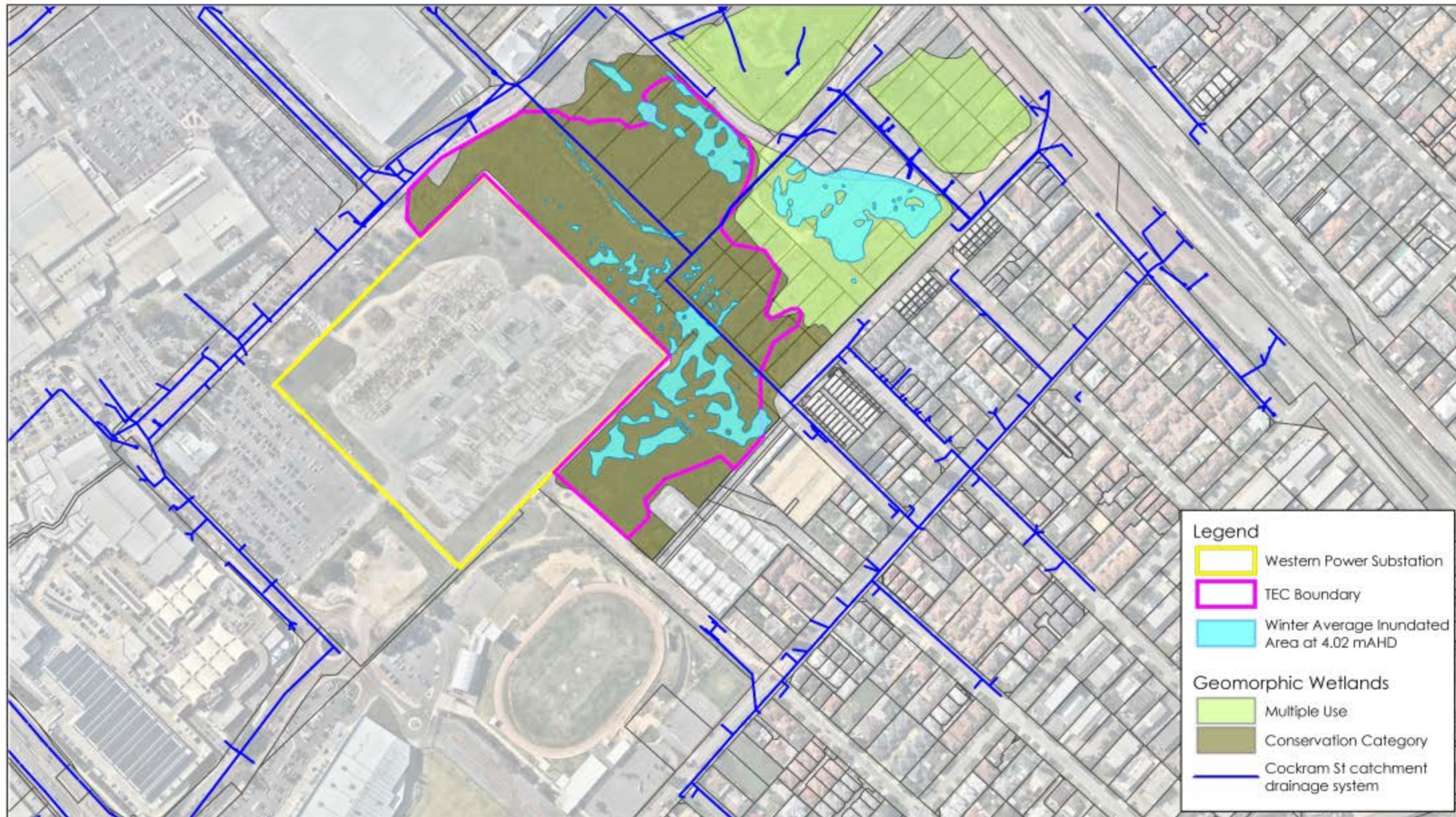
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Figure 4 - Existing drainage, water levels and extent



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3 PRELIMINARY REVIEW AND SITE INSPECTION

The following is a summary of the findings of the site inspections and the desktop review of the proposed road design.

3.1 Onsite groundwater monitoring

As mentioned in Section 2.4, groundwater monitoring program and site inspections were undertaken by Urbaqua staff between July 2017 and June 2018. During August, September and October 2017, inundation was observed at some areas of the Swamp. Figure 5 illustrates the inundation around bore CS-U3 at the corner of the Western Power Sub Station in October 2017. The groundwater has been below the ground level at the other sampling bores during the monitoring period.



Figure 5: Inundation around bore CS-U3 (looking north-west)

3.2 Review of proposed construction

The potential impacts to the Cannington Swamp TEC and CCW resulting from construction of the proposed Southern Link Road are discussed in the sections below.

3.2.1 Post development drainage system

In order to maintain the existing hydrology within the swamp, no runoff from the proposed road will be directed to the swamp. Formal drainage system will be installed for the proposed road to direct its runoff to the existing drainage systems. Based on the local topography, the road surface will be divided into two catchments as shown in Figure 6.

In order to determine post development flows from the proposed road, hydrologic and hydraulic modelling was undertaken with the model XP-Storm. A multi-storm analysis was conducted to determine the critical duration event that produces the largest peak discharge from the modelled catchments. The rainfall used for the modelling is based on 2016 IFD data (BoM, 2018b). The peak flows discharging from each section of the road are provided in Figure 6 and Table 4. There is no external catchment draining to the road.

Table 4: Proposed Southern Link Road Stage 2 post development flows

Discharge location	Catchment Area (ha)	20% AEP (5yr ARI)		1% AEP (100yr ARI)	
		Flow Rate (m ³ /s)	Critical Duration	Flow Rate (m ³ /s)	Critical Duration
1 (to Grey St drainage system)	0.23	0.05	15 min	0.09	10 min
2 (to Liege St drainage system)	0.41	0.08	15 min	0.13	10 min

The modelling results indicate that post development flow rates from the proposed road are relatively small and are not expected to cause substantial erosion effects. Underground pipes can be designed to convey runoff from up to the 20% AEP event downstream to provide for appropriate serviceability. Extreme flooding events (up to the 1% AEP event) that exceed the capacity of pipes can be directed through overland flow on the road.

3.2.2 Post development water balance at the swamp

The water balance for the swamp's existing condition was updated with the proposed road to assess the post development levels within the swamp. Similar to the existing conditions, there will be no upstream catchment discharging to the Cannington Swamp. Runoff from the new road is assumed be directed to the existing drainage systems (Section 3.2.1).

Post development details of the swamp and the winter average water levels during the model period (10-years) are provided in Table 5 and post development inundation area is illustrated in Figure 7. Post-development water balance model details are provided in Appendix C.

Table 5: Post development water balance modelling inputs and results

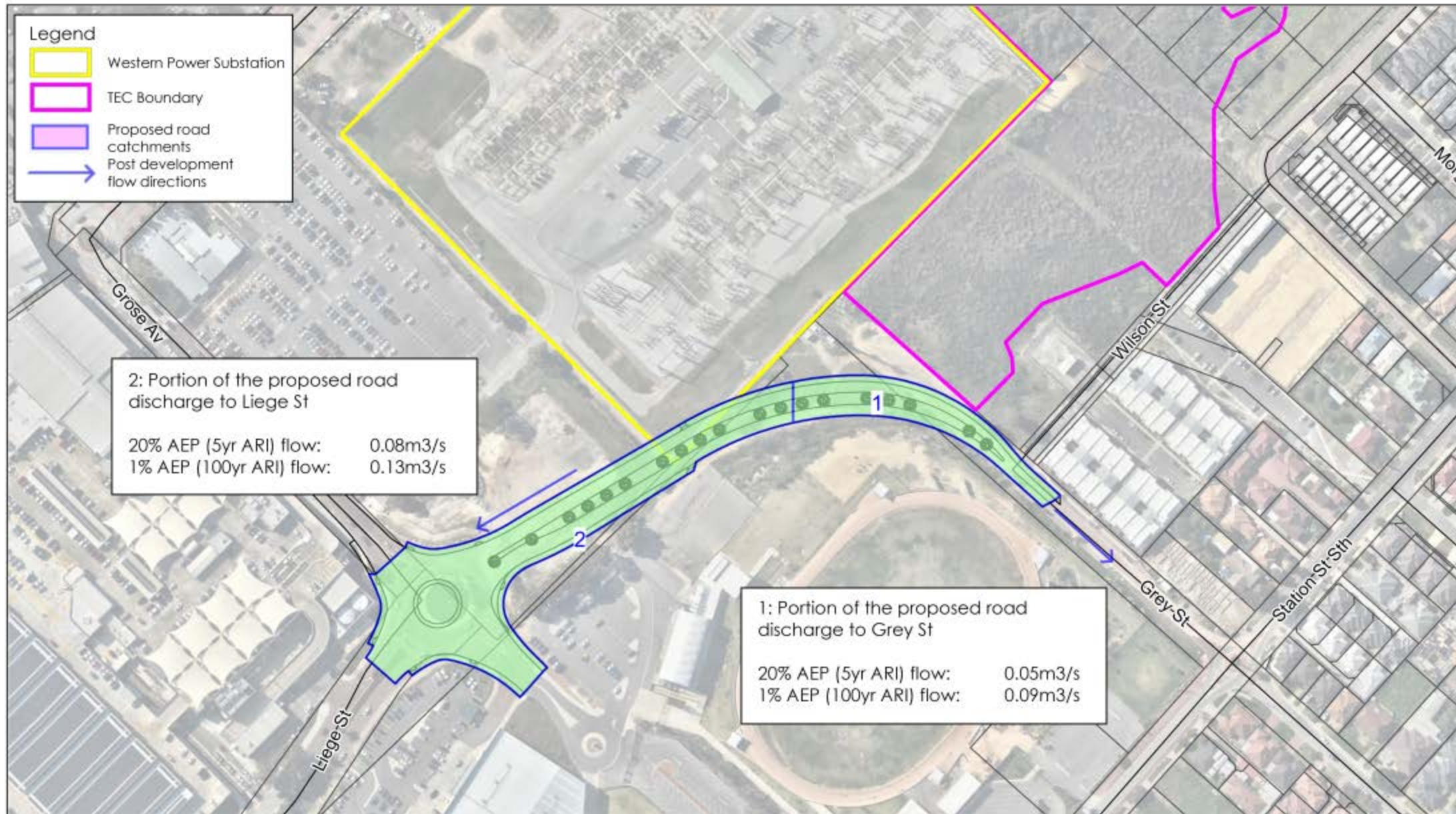
Swamp invert (mAHD)	Base Area (ha)	Overflow level (mAHD)	Total Storage (ML)	Max GW only level (mAHD)	Winter average levels (last 10yrs) (mAHD)
3.50	0.09	4.75	65.6	3.83	4.02

3.2.3 Post development water quality

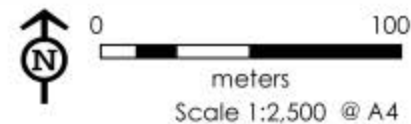
Runoff from the proposed road is not expected to discharge to the swamp, therefore treatment area/swale is not proposed within the road reserve.

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Figure 6 - Proposed road drainage

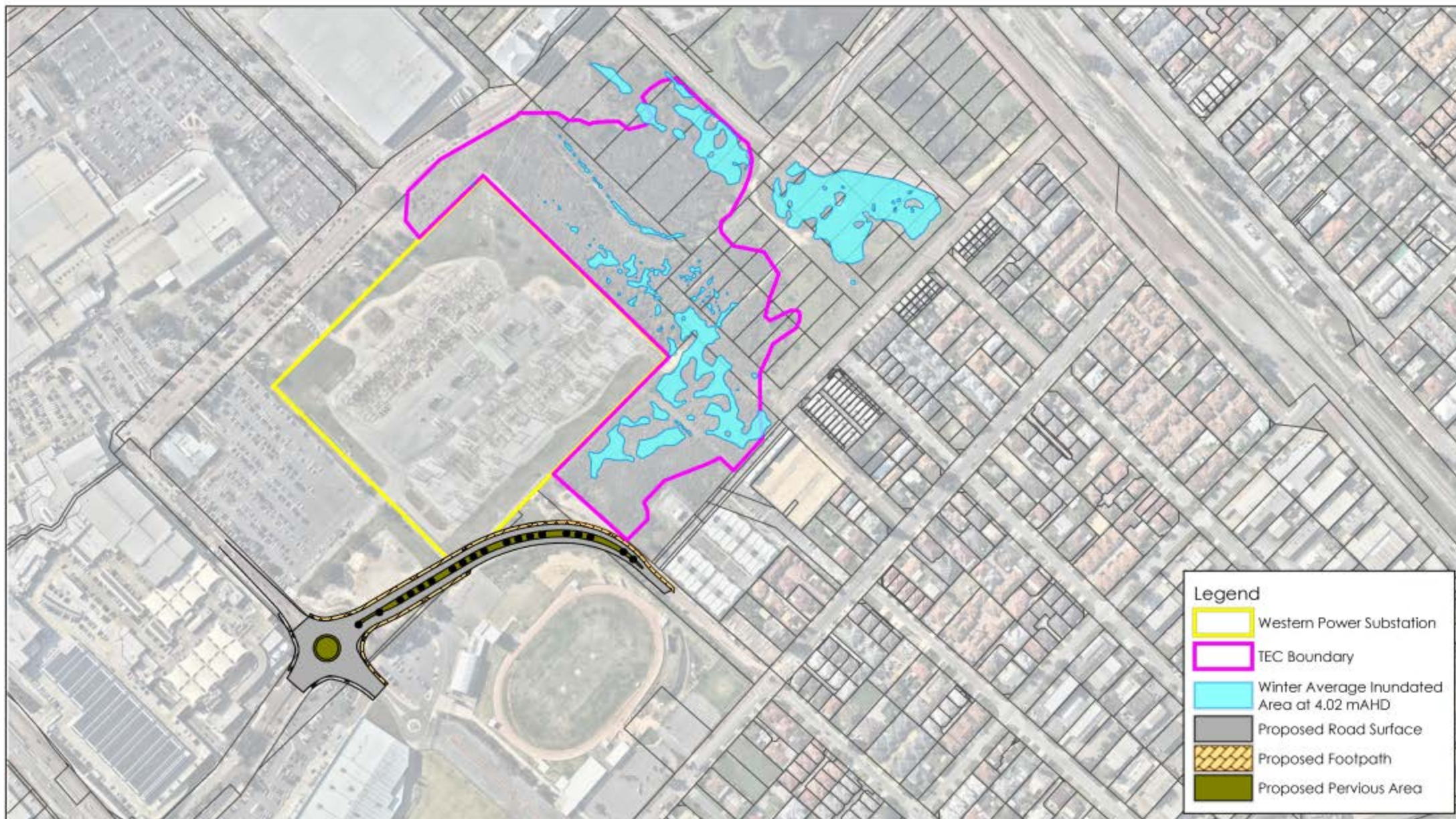


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Figure 7 - Post development water levels and extent



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4 ASSESSMENT OF POTENTIAL IMPACTS

As discussed in Section 2, the critical issues for assessment of environmental impact that were considered by the preliminary review and site inspection are:

- Excavation and filling works – impact on wetland hydrology and water levels;
- Road drainage discharge treatment – impacts to downstream water quality;
- Road compaction – impacts to groundwater flows/levels; and
- Clearing and physical disturbance – impact on flora/fauna and biodiversity values.

4.1 Wetland hydrology and water level impacts

4.1.1 *Post development drainage system*

As shown in Section 3.2, a formal drainage system including underground pipes and overland flow on the road will direct all the runoff from the proposed road to the existing drainage systems at Grey Street and Liege Street existing drainage systems and no stormwater from the proposed road will flow to the swamp. The proposed road construction will be undertaken outside of the Cannington Swamp boundaries and post development hydrology and water levels at the swamp will remain consistent with the existing condition.

4.1.2 *Post development water balance for the swamp*

Water balance model was developed to consider the response of the swamp water levels to the road construction. The post development inundation area is illustrated in Figure 7.

A comparison of the estimated water balance for the proposed and existing systems indicates no change in the water depth or extent within the swamp. The proposed road construction will not interfere with the Cannington Swamp catchment and none of the runoff from the proposed road will be directed to the swamp, as such, the post development water balance at the swamp will be similar to the existing condition.

Construction of the proposed road will result in no change to surface water levels in the TEC and CCW when compared to existing conditions.

4.2 Water quality impacts

Runoff generated by frequent rainfall events has the potential to mobilise pollutants within the catchment. It is proposed that runoff generated by the first 15mm of rainfall onto the new road reserve will be managed in median swales within the road reserve while runoff from larger events will be conveyed directly into the existing downstream drainage system.

Runoff from the proposed road will not discharge into the Cannington Swamp area. Therefore, any change in downstream water quality will not affect the TEC or CCW.

4.3 Groundwater impacts

The proposed Southern Link Road Stage 2 is in an area where the groundwater gradient is quite flat (approximately 1:750) and sloped to the west as can be observed in Figure 3. This indicates that groundwater in the area flows slowly towards the Canning River along a similar alignment to the proposed road reserve and therefore it is considered highly unlikely that the proposed road will present any obstruction to these flows.

4.4 Flora/fauna and biodiversity impacts

The extent of clearing and compaction is entirely outside of the TEC and CCW boundaries. Therefore, the proposed road construction will have no impact on the biodiversity values of the TEC or CCW. However, an application for clearing of native vegetation outside of the TEC and CCW for the proposed road construction is required and is in preparation.

4.5 Risk of potential impacts

Table 6 provides the risk of potential impacts from the construction of the Southern Link Road Stage 2. The proposed development has been considered to maintain a minimal risk of impact on the wetland.

Table 6: Relative risk of impacts for construction of the Southern Link Road Stage 2

Potential impact	Relative risk
Excavation and filling works – impact on wetland hydrology and water levels	Very low
Road drainage discharge treatment – impacts to downstream water quality	Very low
Road compaction – impacts to groundwater flows/levels;	Very low
Clearings and physical disturbance – impact on flora/fauna and biodiversity values	Very low

4.6 Management of construction phase impacts

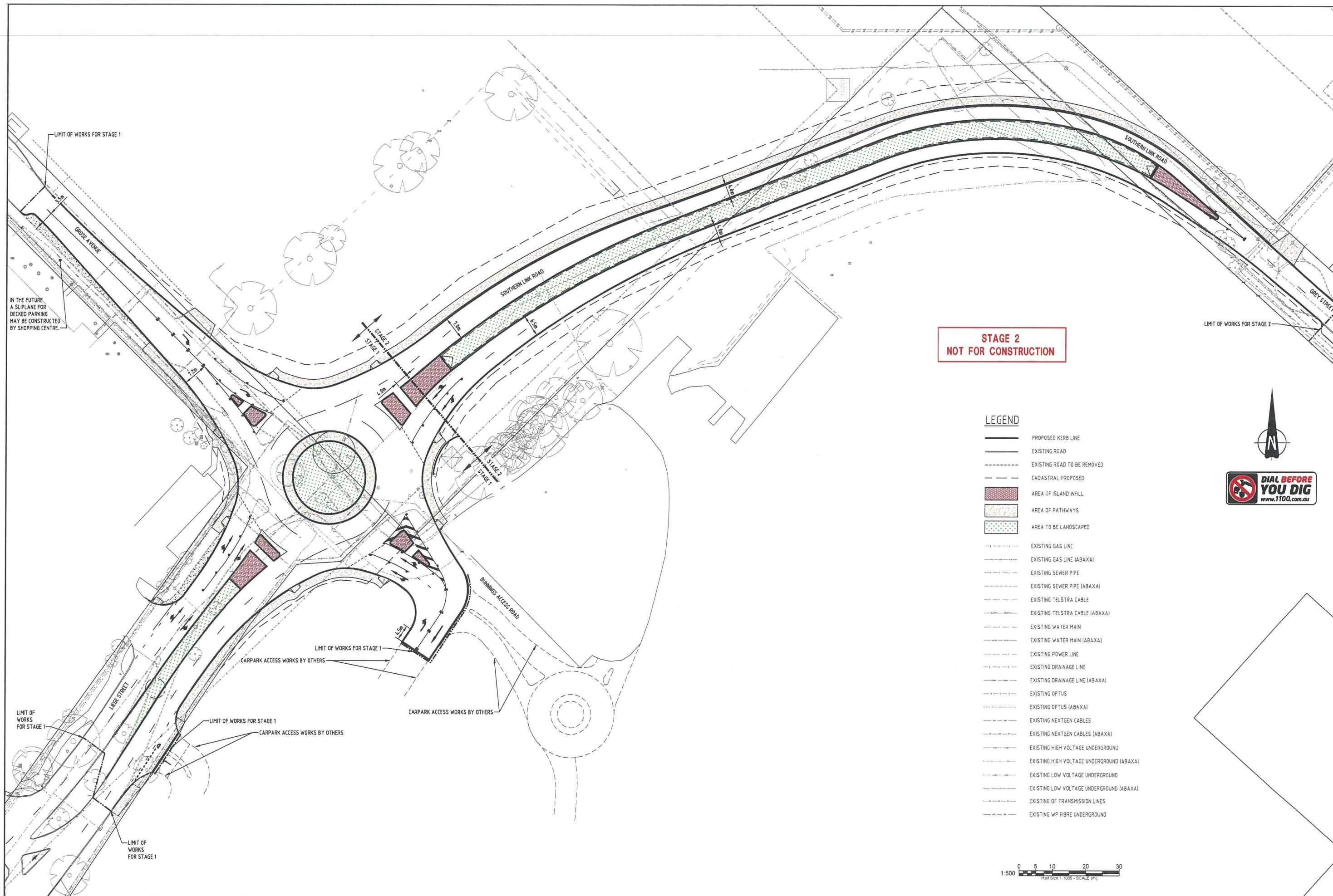
In addition to the long-term risks to the wetland previously discussed in this report, it is important to consider the short-term risks presented by construction activity at the wetland boundary. The following strategies are recommended for consideration in developing an appropriate construction environmental management plan:

- Construction traffic should be contained outside of the wetland boundaries;
- Construction during periods of low groundwater is preferred to avoid requirements for any dewatering;
- Construction during periods of low rainfall is preferred to avoid dispersal of sediment and construction materials into the wetland;
- Sediment fencing should be provided along the edge of the construction area to provide protection from wind and water borne sediment and construction materials; and
- Temporary stockpiles should be located outside the wetland boundaries and contained by sediment fencing.

5 REFERENCES AND RESOURCES

- Australian and New Zealand Environment and Conservation Council and Agriculture and Resource Management Council of Australia and New Zealand (ANZECC & ARMCANZ) 2000, *National Water Quality Management Strategy: Australian and New Zealand Guidelines for Fresh and Marine Water Quality*, Paper No. 4, Volume 1, October 2000.
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APPENDIX A – ENGINEERING DRAWINGS OF THE PROPOSED WORK



**STAGE 2
NOT FOR CONSTRUCTION**

LEGEND

- PROPOSED KERB LINE
- EXISTING ROAD
- EXISTING ROAD TO BE REMOVED
- CADASTRAL PROPOSED
- AREA OF ISLAND INFILL
- AREA OF PATHWAYS
- AREA TO BE LANDSCAPED
- EXISTING GAS LINE
- EXISTING GAS LINE (ABAXA)
- EXISTING SEWER PIPE
- EXISTING SEWER PIPE (ABAXA)
- EXISTING TELSTRA CABLE
- EXISTING TELSTRA CABLE (ABAXA)
- EXISTING WATER MAIN
- EXISTING WATER MAIN (ABAXA)
- EXISTING POWER LINE
- EXISTING DRAINAGE LINE
- EXISTING DRAINAGE LINE (ABAXA)
- EXISTING OPTUS
- EXISTING OPTUS (ABAXA)
- EXISTING NEXTGEN CABLES
- EXISTING NEXTGEN CABLES (ABAXA)
- EXISTING HIGH VOLTAGE UNDERGROUND
- EXISTING HIGH VOLTAGE UNDERGROUND (ABAXA)
- EXISTING LOW VOLTAGE UNDERGROUND
- EXISTING LOW VOLTAGE UNDERGROUND (ABAXA)
- EXISTING OF TRANSMISSION LINES
- EXISTING WP FIBRE UNDERGROUND



PROJECT: SOUTHERN LINK ROAD CANNINGTON		COPYRIGHT COPYRIGHT IN THIS DRAWING IS THE PROPERTY OF THE CONSULTANT. THE CLIENT HAS LICENSE TO USE THIS DRAWING FOR THE PROJECT. ONLY THE USER SHALL BE RESPONSIBLE FOR "SITE CHECKING" ALL DRAWINGS BEFORE COMMENCEMENT OF WORK. CAD DRAWINGS DO NOT MANUALLY ALTER. ONLY PLANS WITH NUMERICAL REVISION (REV. 1) OR HIGHEN AND SIGNED AS APPROVED SHALL BE USED FOR CONSTRUCTION.		Porter Consulting Engineers Level 2 Midland Court 38 Midland Road PO Box 453 WA PO Box 9036 Canning Bridge 6153 WA Tel (08) 9315 1955 Fax (08) 9315 1955 Email: office@portereng.com.au www.portereng.com.au		CLIENT: CITY OF CANNING	DRAWING LAYOUT PLAN STATUS: FOR CONSTRUCTION	SCALE: 1:500 DATE: AUG 2016 DESIGN: ELW DRAWN: MJV CHECK: APPD	DRAWING NO. 16-4-49/100 FILE NAME: S:\ACTIVE PROJECTS\16-4-49\ACAD\16-4-49-100.dwg	REV NO. 0	DESIGN DRAWING SIZE A1
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APPENDIX B – WATER BALANCE MODEL – EXISTING CONDITION

WATER BALANCE CALCULATIONS

Lagoon Water Balance - Options Analysis

Calculation Sheet 1



Scenario Existing Condition

Inf Model Aquifer connected

Rainfall Data

Source

Reference 9106

Climate Data

Date	Rainfall mm	Month	Pan Evap mm/month	mm/day	Surr GWL m AHD
1/01/2008	0	Jan	297	9.6	2.80
2/01/2008	0	Feb	257	8.9	2.52
3/01/2008	0	Mar	224	7.2	2.33
4/01/2008	0	Apr	123	4.1	2.33
5/01/2008	0	May	87	2.8	2.30
6/01/2008	0	Jun	59	2.0	2.95
7/01/2008	0	Jul	60	1.9	3.21
8/01/2008	0	Aug	69	2.2	3.83
9/01/2008	0	Sep	106	3.5	3.68
10/01/2008	0	Oct	154	5.0	3.53
11/01/2008	0	Nov	203	6.8	3.13
12/01/2008	0	Dec	259	8.4	2.80
13/01/2008	0	Jan	297	9.6	2.80125

Lake Surface Area Contours

Level	Area (m2)	Perimeter	Storage
3.5	875	167.633023	0
3.75	13257	652.496488	1767
4	43833	1186.46832	8903
4.25	73421	1535.55614	23560
4.5	85999	1661.88991	43487
4.75	90630	1706.04916	65566
5	91349	1712.80314	88313
5.25	91370	1713	111153

Model Results	Total (ML)
Change in Storage	0.9

INPUTS	Peak Annual
Direct Rainfall	252.4 41.1
Catchment Runoff	93.0 12.9
net inflow/outflow	0.0 0.0

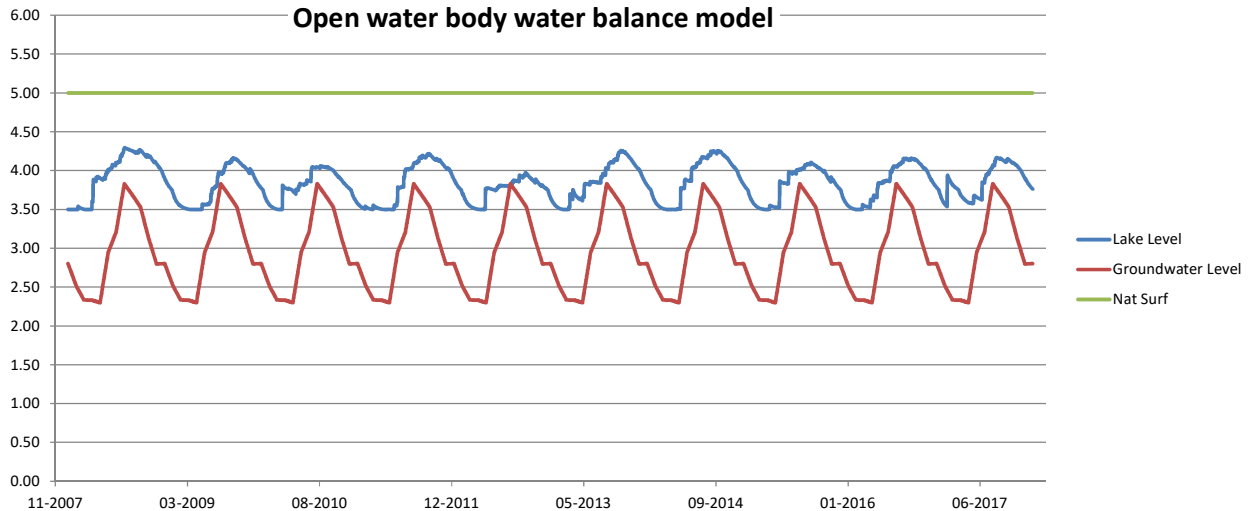
OUTPUTS	
Evaporation	342.1 45.6
Net seepage to GW	2.3 22.4
Overflow as Stormwater	0.0 0.0
Low flow discharge	0.0 0.0

Annual Inflow	ML
Maximum	0.0
Minimum	0.0
Average	0.0

Model Inputs

initial water level	2.80	mAHD
pan evaporation factor	0.75	El/Ep
aquifer conductivity - 10^A		m/s
aquifer conductivity	0.008	m/day
distance of influence	150	m
base of aquifer	-15	m
base of lake	3.5	mAHD
depression storage	15	mm
natural surface level	5	mAHD
site area draining to lake	9.137	ha
overflow level	4.75	mAHD
max volume	65565.625	m3
Runoff parameter	100.00%	
Low flow discharge rate		L/s
Drain invert		mAHD

Open water body water balance model



24/02/2008 0

APPENDIX C – WATER BALANCE MODEL – POST DEVELOPMENT CONDITION

WATER BALANCE CALCULATIONS

Lagoon Water Balance - Options Analysis

Calculation Sheet 1



Scenario Post Development Condition

Inf Model Aquifer connected

Rainfall Data

Source

Reference 9106

Climate Data

Date	Rainfall mm	Month	Pan Evap mm/month	mm/day	Surr GWL m AHD
1/01/2008	0	Jan	297	9.6	2.80
2/01/2008	0	Feb	257	8.9	2.52
3/01/2008	0	Mar	224	7.2	2.33
4/01/2008	0	Apr	123	4.1	2.33
5/01/2008	0	May	87	2.8	2.30
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11/01/2008	0	Nov	203	6.8	3.13
12/01/2008	0	Dec	259	8.4	2.80
13/01/2008	0	Jan	297	9.6	2.80125
14/01/2008	0				
15/01/2008	0				
16/01/2008	0				
17/01/2008	0				
18/01/2008	0				
19/01/2008	0				
20/01/2008	0				
21/01/2008	0				
22/01/2008	0				
23/01/2008	0				
24/01/2008	0				
25/01/2008	0				
26/01/2008	0				
27/01/2008	0				
28/01/2008	0				
29/01/2008	0				
30/01/2008	0				
31/01/2008	0				

Model Inputs

initial water level	2.80	mAHD
pan evaporation factor	0.75	El/Ep
aquifer conductivity - 10 ⁴		m/s
aquifer conductivity	0.008	m/day
distance of influence	150	m
base of aquifer	-15	m
base of lake	3.5	mAHD
depression storage	15	mm
natural surface level	5	mAHD
site area draining to lake	9.137	ha
overflow level	4.75	mAHD
max volume	65565.625	m3
Runoff parameter	100.00%	
Low flow discharge rate		L/s
Drain invert		mAHD

Lake Surface Area Contours

Level	Area (m2)	Perimeter	Storage
3.5	875	167.633023	0
3.75	13257	652.496488	1767
4	43833	1186.46832	8903
4.25	73421	1535.55614	23560
4.5	85999	1661.88991	43487
4.75	90630	1706.04916	65566
5	91349	1712.80314	88313
5.25	91370	1713	111153

Model Results

Change in Storage	Total (ML)
	0.9

INPUTS

	Peak Annual
Direct Rainfall	252.4
Catchment Runoff	93.0
net inflow/outflow	0.0

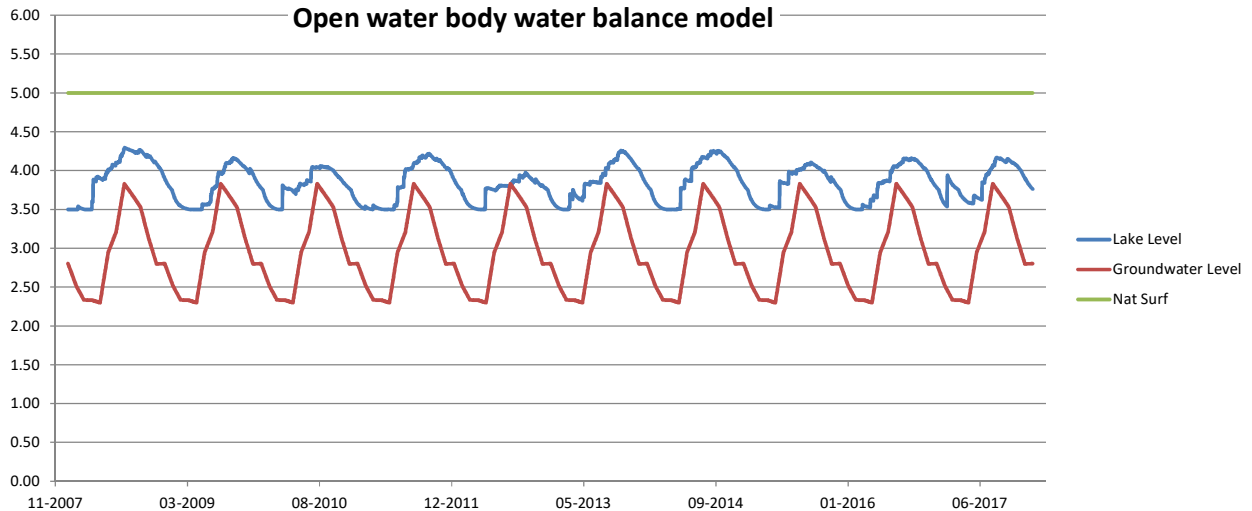
OUTPUTS

Evaporation	342.1	45.6
Net seepage to GW	2.3	22.4
Overflow as Stormwater	0.0	0.0
Low flow discharge	0.0	0.0

Annual Inflow

	ML
Maximum	0.0
Minimum	0.0
Average	0.0

Open water body water balance model



24/02/2008 0



Client: City of Canning

Report	Version	Prepared by	Reviewed by	Submitted to Client	
				Copies	Date
Preliminary draft	V1	AN	HB	Electronic	13/11/2018
Final report	V2	AN	HB	Electronic	15/11/2018
Minor revisions	V3	HB	HB	Electronic	21/11/2018

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